

# Public Comments Report

IS-HRC First Public Comments Draft Dated November 2006

## Item: Barbera 05

Section: **101.1**

Committee Action: **Negative**

Comment: The reference to seismic is inappropriate in the context of this standard.

## Item: Shackelford 01

Sections: **101.2 through 101.6**

Committee Action: **Accept**

Comment:

## Item: Herrenbruck 02

Sections: **101.2 and 701.2**

Committee Action: **Negative**

Comment: The proponent did not provide specific text for committee consideration. However, the committee has compiled the standard within the scope that was given to the committee.

## Item: Humphreys 01

Figures: **102A and 102B**

Committee Action: **Accept**

Comment:

## Item: Schmid 01

Figures: 102A, 102B, 102C and 103A

Committee Action: Negative

Comment: The proposals are related to issues contained in the AF&PA Wood Frame Construction Manual (WFCM). Acceptance would create a conflict between this standard and the WFCM.

## Item: Barbera 01

Section: 102.2

Committee Action: Accept in principle

Comment: Modify Section 102.1 as follows, and delete Section 102.2.

1. Change Sections 102.1 as follows:

### 102 DESIGN PARAMETERS

#### 102.1 Generic Building Geometry

The provisions of this standard apply to enclosed wood or steel framed, concrete, masonry and insulated concrete form (ICF) walled residential buildings formed by rectangular shaped elements in plan view and having the geometry shown in Table 102.1.

**102.1.1 Enclosed exterior walls.** The requirements are based on all exterior walls having solid elements (walls, windows, and doors) for the full perimeter of the building. Open porches not exceeding 20 feet in width and constructed in accordance with Sections 208 and 308 shall be permitted.

**102.2.2 Non-rectangular buildings** Nonrectangular shaped buildings in plan view shall be permitted in accordance with the provisions of Section 105

2. Delete Section 102.2 Open Structures and Figures

## Item: Shackelford 02

Section: 102.2

Committee Action: Accept in principle

Comment: See Barbera 01

**Item: Low 01/Low 02**

**Section: 102.3**

**Committee Action: Low 01 – Accept  
Low 02 -- Accept**

**Additional motion:** Low 02 – Add comment that these sections will need to be made applicable to steel framing.

**Comment:** The last sentence of Low 01 is to be deleted because it is commentary and not needed.

**Item: Humphreys 02**

**Section: 103**

**Committee Action: Accept**

**Comment:**

**Item: Humphreys 03**

**Section: 202.1.3, 202.1.4 and App. A**

**Committee Action: Negative**

**Comment:** Non-persuasive. This standard continues to be referenced in the IRC at this time. The proponent did not provide a compelling reason to omit it.

**Item: Ehrlich 01**

**Section: 202.1.7.1**

**Committee Action: Accept**

**Comment:**

**Item: Humphreys 04**

**Section: 202.1.7.1**

**Committee Action: Negative**

**Comment:** The proposed text is not necessary.

**Item: Kurtz 01**

**Section: 202.1.7.1**

**Committee Action: Accept**

**Comment:** Change ref. to Table R602.3(1)

**Item: Kurtz 02**

**Section: 202.1.7.1**

**Committee Action: Negative**

**Comment:** The committee continues to maintain that staples are not acceptable fasteners to use for buildings in high wind regions.

**Item: Kurtz 03**

**Section: 202.1.7.1**

**Committee Action: Accept in principle**

**Add to 202.1.7.2 “**

**“Nails shall conform to the requirements of ASTM F1667 including supplementary requirements.”**

**Comment:**

**Item: Kurtz 08 Withdrawn**

**Section: 202.1.7.1 and 304.2**

**Committee Action:**

**Comment:**

**Item: Kurtz 04**

**Section: 202.1.7.2**

**Committee Action:** Accept in principle –

**Comment:** Add Appendix E per John Kurtz suggestion:

**Appendix E**  
Dimensions of Nails Described by Pennyweight System

Pennyweight Description	Length, inch	Diameter, inch
6d common	2	0.113
8d common	2-1/2	0.131
10d common	3	0.148
16d common	3-1/2	0.162
6d box	2	0.099
8d box	2-1/2	0.113
10d box	3	0.128
16d box	3-1/2	0.135

**Item: Graber 01**

**Figures: 202A and 202B**

**Committee Action:** Accept

**Comment:**

**Item: Graber 02**

**Table: 202A**

**Committee Action:** Accept

**Comment:**

**Item: Graber 03**

**Tables: 203, 204 and 205**

**Committee Action:** Accept

**Comment:**

## Item: Humphreys 05

### Section: 204

**Committee Action:** Accept in principle

**Comment:** Revise Section 204.3.8 as follows:

#### 204.3.8 Connections for Masonry Walls in Accordance with 205

1. Bearing ends of joists or trusses shall be connected to masonry walls by a ledger bolted to the wall as shown in ~~Table 204E1 and~~ Figure 204F1. Anchor bolts shall be sized and spaced in accordance with Table 204E and with a bolt located not less than 6 inches nor greater than 12 inches from each end of each ledger member. The ledger shall be No. 2 Southern Pine or No. 2 Douglas Fir. Framing shall be fastened to the ledger with metal joist hangers properly rated for all gravity loads. Floor sheathing shall be fastened to the ledger the same as to other floor framing members.
2. Where the masonry wall above the floor line is thinner than the wall below, first-story wood floors may bear on and be attached to the top of the wall below as shown for stemwalls in Figure 203D. Framing shall be fastened to the plate in accordance with Table R602.3(1) of the *International Residential Code*. The plate shall be bolted to the wall with 1/2" diameter anchor bolts spaced at a maximum of 6 feet on center, with a bolt located not less than 6 inches nor greater than 12 inches from each end of each plate member and embedded a minimum of 7 inches into the masonry wall

## Item: Kurtz 05

### Section: 204.3.7, 204.4 and 207.5.1

**Committee Action:** Accept in principle Pass

**Comment:** Refer to IBC Tables 2306.3.1 and 2306.3.2 Also correct in other locations . See p. 178, 180, 183, 184 in App. D.

## Item: Kurtz 06

### Section: 204.4 and 207.5.1

**Committee Action:** Negative

**Comment:** The committee felt that the proposed language would have the effect of implying that fasteners such as staples could be used.

## Item: Graber 04

### Tables: 204G and 204H

#### Committee Action: Accept in Principle

#### Motion – use “Tributary” Pass U

#### Comment: Modify the standard as follows:

Revise sentence of section as follows:

Floor sheathing and fasteners shall be capable of resisting the total shear loads specified in Tables 204G and 204H for the applicable ~~location in the building and the distance between shear walls~~ exposure and wind speed.  
(Remainder of Paragraph unchanged).

Delete Tables ~~205G-204G~~ and ~~205-204~~ H (including footnotes) and replace with new Table ~~205G~~204G:

**TABLE 204G**

**TOTAL FLOOR DIAPHRAGM SHEAR LOAD<sup>1,2</sup> LBS PER SIDE**

Exposure	Wind Speed	Perpendicular Dimension <sup>3</sup> , ft							
		24	32	40	48	56	64	72	80
B	100	1893	2467	3039	3608	4178	4759	5354	5949
	110	2291	2986	3677	4366	5055	5759	6479	7198
	120	2727	3553	4376	5196	6016	6853	7710	8567
	130	3200	4170	5135	6098	7060	8043	9049	10054
	140	3711	4836	5956	7073	8188	9328	10494	11660
	150	4260	5552	6837	8119	9399	10708	12047	13386
C	100	2655	3459	4260	5059	5857	6673	7507	8341
	110	3212	4186	5155	6122	7087	8074	9083	10093
	120	3823	4981	6135	7285	8434	9609	10810	12011
	130	4486	5846	7200	8550	9898	11277	12687	14096
	140	5203	6780	8350	9916	11480	13079	14713	16348
	150	5973	7784	9586	11383	13178	15014	16890	18767

NOTES:

1. Loads are for walls/diaphragm edges parallel to the direction of the wind and are based on tributary wall height of 10' ~~wall height. Multiply by 0.8 for 8' wall heights. For tributary wall heights other than 10 ft multiply by tributary wall height (ft)/10.~~
2. To determine required individual connector load between the wall and the diaphragm, divide the tabular shear load by the number of connectors.
3. Dimension of diaphragm perpendicular to wall on which connectors are being designed.

## Item: Humphreys 06

### Section: 205.2.6

#### Committee Action: Negative

**Comment:** The method chosen by the committee will allow interpolation.

**Item: Humphreys 07**

**Section: 205.3**

**Committee Action: Accept in principle**

**Comment:** Add "Bond beams shall contain one #4 bar minimum." at the end of Section 205.2.1

**Item: Humphreys 08**

**Section: 205.3.4**

**Committee Action: Accept**

**Comment:**

**Item: Ehrlich 02**

**Section: 205.3.7**

**Committee Action: Accept**

**Comment:**

**Item: Ehrlich 03**

**Sections: 205.4.1 and 205.4.4**

**Committee Action: Accept**

**Comment:**

**Item: Ehrlich 04**

**Section: 205.5.4**

**Committee Action: Accept**

**Comment:**

**Item: Humphreys 09**

**Tables: 205D, 205E1 and 205E2**

**Committee Action: Negative**

**Comment:** See Humphreys 06

**Item: Humphreys 10**

**Table: 205D**

**Committee Action: Accept in principle**

**Comment:** See Graber 05

**Item: Graber 05**

**Table: 205D**

**Committee Action: Accept**

**Comment:**

**Item: Graber 06**

**Figure: 205E**

**Committee Action: Accept**

**Comment:**

**Item: Graber 07**

**Figure: 205E2**

**Committee Action: Accept**

**Comment:**

**Item: Graber 08**

**Figure: 205E3**

**Committee Action: Accept**

**Comment:**

**Item: Graber 09**

**Figure: 205E4**

**Committee Action: Accept**

**Comment:**

**Item: Graber 10**

**Table: 205F1**

**Committee Action: Accept**

**Comment:**

**Item: Barbera 02**

**Tables: 205G and 205H**

**Committee Action: Accept in principle**

Also include a figure from Florida Building code – end wind zone

**Comment:** See Graber 10

**Item: Graber 11**

**Table: 205G2**

**Committee Action: Accept**

Table to be labeled 205G2. Move footnote 1 from column heading “Perpend. to wall  $F_2$ ” to column heading “Connector load, lbs”.

**Comment:**

**Item: Graber 12**

**Table: 205G3**

**Committee Action: Accept**

**Comment:**

**Item: Everly 01**

**Tables: 205H1, 205H2 and 205H3**

**Committee Action: Accept**

**Comment:**

**Item: Everly 02**

**Tables: 205J1, 205J2 and 205J3**

**Committee Action: Accept**

**Comment:**

**Item: Graber 13**

**Table: 205J3**

**Committee Action: Accept**

**Comment:**

**Item: Everly 03**

**Tables: 205S1 and 205S3**

**Committee Action: Accept**

**Comment:**

**Item: Everly 04**

**Table: 205S2**

**Committee Action: Accept**

**Comment:**

**Item: Ehrlich 05**

**Section: 207.2**

**Committee Action: Accept**

**Comment:**

## Item: Kurtz 07

**Sections: 207.3.3, 502.4.5, 504.3.7.2, 507.7.1.1, 504.8.7.1.1 and Table 504.3.5**

**Committee Action:** Accept in principle.

**Comment:** Modify Section 207.3.3 as follows.

**207.3.3 Sheathing Fastenings:** Sheathing shall be fastened to roof framing with 8d ring-shank nails at 6 inches o.c. at edges and 6 inches o.c. at intermediate framing. Ring-shank nails shall have the following minimum dimensions:

1. 0.113 inch nominal shank diameter
2. Ring diameter of 0.012 over shank diameter
3. 16 to 20 rings per inch
4. 0.280 inch full round head diameter
5. 2 inch nail length

EXCEPTIONS: (See Figure 207B for nailing zones)

1. Where Group III species framing lumber is used, spacing of ring-shank fasteners shall be 4 inches o.c. in nailing zone 3 for 130 mph or greater design wind speeds.
2. Where Group III species framing lumber is used, spacing of ring-shank fasteners shall be permitted at 12 inches o.c. at intermediate framing in nailing zone 1 for any design wind speed and in nailing zone 2 for 110 mph or lower design wind speeds.
3. Where Group II species framing lumber is used, spacing of ring-shank fasteners shall be permitted at 12 inches o.c. at intermediate framing in nailing zones 1 for any design wind speed and in nailing zone 2 for 120 mph or lower design wind speeds.
4. Where Group II species framing lumber is used, 8d common or 8d hot dipped galvanized box nails at 6 inch o.c. at edges and 6 inch o.c. at intermediate framing shall be permitted for 100 mph or lower design wind speeds.
5. Where diaphragm requirements necessitate a closer nail spacing.

**Item: Ehrlich 06**

**Sections: 207.6.2, 209.2.2.2, 404.5.1 and 404.5.2**

**Committee Action: Accept in principle**

**Comment:** Add ASTM F1554 in 207.6.2, 404.5.1, 404.5.2 (“either” “or”) no change to 209.2.2.2. Change “Grade” to “ASTM”

Section R611 of the IRC presently permits A307 or F1554. There is no technical reason to be inconsistent with IRC and not allow the same alternatives in this standard.

**Item: Barbera 03**

**Section: 208**

**Committee Action: Negative**

**Comment:** The suggested changes are not appropriate in Section 208.

**Item: Shackelford 03**

**Sections: 302.1.5 and 307**

**Committee Action: Negative**

**Comment:** While the comment’s intent is understood, the proposed wording could be interpreted to permit elimination of all uplift anchorage required by the WFCM, including tie-downs, etc. Suggest that the commenter come back with revised language.

**Item: Kelly 01**

**Section: 304.1**

**Committee Action: Accept in principle**

**Comment:** Use the term “Registered Design Professional” rather than “licensed professional.”

## Item: Kurtz 09

### Section: 304.3

#### Committee Action: Accept in principle

**Comment:** The commenter did not provide strike out and underline wording. See the following proposed wording.

**304.3** Fasteners and connectors that are exposed directly to the weather or subject to salt corrosion in coastal areas, as determined by the building official, shall comply with the following:

304.3.1 ~~Metal plates, connectors, screws, bolts and nails exposed directly to the weather or subject to salt corrosion in coastal areas, as determined by the building official, shall be of stainless steel, or hot dipped galvanized to meet the requirements of ASTM A153 Class D for fasteners with diameters of 3/8" or less, ASTM A153 Class C for fasteners with diameters over 3/8", or ASTM A653 for connectors, after the fastener or connector is fabricated to form a zinc coating not less than 1 oz per sq ft, or hot dipped galvanized with a minimum coating of 1.8 oz per sq ft of steel meeting the requirements of ASTM A 90 Triple Spot Test.~~

304.3.2 Metal plates and connectors shall be stainless steel, hot dipped galvanized prior to fabrication to meet ASTM A653 Coating Designation G185, or hot dipped galvanized after fabrication to meet ASTM A123 Coating Grade 45.

## Item: Ehrlich 07

### Table: 305A

**Errata:** Add the word stem to the Table as shown below:

**TABLE 305A  
MINIMUM FOUNDATION DIMENSIONS**

Figure	Foundation Type	Minimum T (inches)	Minimum W (inches)
305B, C, D	Stem wall	10	20
305E	Solid or hollow masonry <u>stem</u> wall <sup>1</sup>	20	12
305F	Monolithic exterior slab on grade footing	20	12
305G	Monolithic interior slab on grade footing	20	12
305H	Stem wall with slab on grade	10	20

<sup>1</sup> Permitted only for basic wind speeds of 100 and 110 mph and one- and two story structures.

**Committee Action: Accept**

**Comment:**

## Item: McGuire 01

### Table: 305A

**Committee Action: Accept**

**Comment:**

**Item: Ehrlich 08**

**Figure: 305C**

**Committee Action:** Accept in principle.

**Comment:** The committee accepts the proposed changes to the vertical bars and the top reinforcing. The top reinforcing is to be located 3" clear from the top of the bond beam to fall approximately mid-height of the bond beam. The committee rejects the depiction of the footing reinforcing because this figure is addressing stemwall reinforcing only.

**Item: Kelly 02**

**Table: 305D1 (Figure 305D1)**

**Committee Action: Negative**

**Comment:** The proposed note does not provide specifics necessary. Alternatives can be allowed in accordance with Section 101.4.

**Reason:**

**Item: Kelly 03**

**Table: 305D2 (Figure 305D2)**

**Committee Action: Negative**

**Comment:** See Kelly 02

## Item: Ehrlich 09

### Section: 306

**Committee Action:** Accept in principle

**Comment:** **Modify as follows:**

**306.1** Concrete floors shall be cast in place slab-on-grade.

**306.2** ~~The top of a monolithic slab on grade shall be at least 8 6 inches above finished grade.~~ The slab shall be not less than 3 1/2 inches thick. The slab shall have 6x6 W1.4 X W1.4 welded wire fabric at mid-height.

~~**306.3**~~ **305.3.4** Post-tensioned slab-on-grade floor systems designed by a registered design professional engineer in accordance with *PTI Design of Post-Tensioned Slabs on Ground* shall be permitted.

Make suggested change to 306.3 and move to 305.3.4. Change “registered professional engineer” to “registered design professional” to be consistent with the IRC.

Not accept changes to 306.2 or figure. Delete the height above grade from 306.2 and the figure since this is specified in the IRC and can vary based on “T” and the “12’ min “ below grade.

## Item: Schmid 02

**Figures:** 305G2 and 305G3

**Committee Action:** Negative

**Comment:** The proposed modification would limit options for design.

## Item: Schmid 03

**Figure:** 307S1

**Committee Action:** Negative

**Comment:** Configuration for 3 x sill would require retesting.

## Item: Ehrlich 10

**Section:** 307.1 and 307.2

**Committee Action:** Accept

**Comment:**

**Item: Shackelford 04**

**Section: 307.1**

**Committee Action: Negative**

**Comment:** The previous testing did not cover the existing parameters (16" stud spacing) in the standard. The additional testing was not completed and not available to the committee.

**Item: Shackelford 05**

**Section: 307.1**

**Committee Action: Negative**

**Comment:** Not necessary. If there is a failure, it will probably occur at the bottom plate.

**Item: Shackelford 06**

**Table: 307S1**

**Committee Action: Negative**

**Comment:** The test data used to support the proposed modification is inappropriate.

**Item: Shackelford 07**

**Tables: 307S1 and 307S2**

**Committee Action: Accept**

**Comment:**

**Item: Shackelford 08**

**Figure: 307S1**

**Committee Action: Accept**

**Comment:**

**Item: Olds 01**

**Section: 308**

**Committee Action: Negative**

**Comment:** The commenter did not provide a specific suggestion for revision to the standard text.

**Item: Grundahl 02**

**Table: 308.2**

**Committee Action: Accept in principle**

**Comment:** Accept the deletion of the word “Unfactored” in footnote. Reject the addition of ASD since this is redundant.

**Item: Grundahl 01**

**Figure: 308C**

**Committee Action: Accept in principle – Accept 1<sup>st</sup> and 2<sup>nd</sup> item. No action on item 3.**

**Comment:** Item 1: Agree to revised wording. Item 2: Say “Uplift load not less than specified on truss design drawing for truss above.” Item 3: Intent is design per WFCM or COFS/PM. Commenter needs to provide specific requested wording.

**Item: Shackelford 09**

**Chapter: 5**

**Committee Action: Accept**

**Comment:**

**Item: Kelly 04**

**Section: 501.2**

**Committee Action: Accept in principle**

**Comment:** Replace “and” with “or” in second line of proposed modification.

**Item: Kelly 05**

**Section: 502.2.1**

**Committee Action: Negative**

**Comment:** The use of the word “approved” in this context implies that there is more for the building official to do beyond the normal review and approval of construction documents. This is not necessary.

**Item: Kelly 06**

**Section: 503.1**

**Committee Action: Accept in principle see --**

**Comment:** Revise 503.1 to read: “Roof sheathing shall be solid sheathing installed in accordance with this standard.”

**Item: Kochheiser 01**

**Sections: 503.2, 504.7.7.1.2 and 504.8.7.1.2**

**Committee Action: Accept in Principle**

**Comment:** See Kurtz 11. The committee opted to simply reference Section 304.3. This requirement should be consistent for all materials.

**Item: Kelly 07**

**Section: 504.1**

**Committee Action: Accept**

**Comment:**

**Item: Kelly 08**

**Section: 504.2**

**Committee Action: Accept**

**Comment:**

**Item: Shackelford 10**

**Section: 504.3.4**

**Committee Action: Accept**

**Comment:**

**Item: Shackelford 11**

**Table: 504.3.4.1**

**Committee Action: Accept in Principle**

**Comment:** Move the heading **Gable Roof** and **Hip Roof** slopes from after **Importance Factor = 1.00** to after the **Basic Wind Speed, V** row.

## **Item: Kurtz 10**

### **Table: 504.3.5**

**Committee Action: Accept in principle**

**Comment: Revise items 1 and 2 to read as follows:**

2. Ring shank nails shall be 10d ring shank corrosion resistant steel ~~nails~~ (with the following minimum dimensions: 3 inches long, 0.283 inch flat head diameter, ~~0.124~~ 0.120 inch undeformed shank diameter, and 0.131 inch ring diameter).

3. Smooth or screw shank nails shall be 10d corrosion resistant steel (with the following minimum dimensions: 3 inches long, ~~0.28~~ 0.283 inch flat head diameter, ~~0.128 inch screw~~ 0.120 inch undeformed shank diameter or 0.131 inch ~~smooth shank screw~~ diameter).

## **Item: Kelly 09**

### **Section: 504.3.8.1**

**Committee Action: Negative**

**Comment: See Kelly 05**

## Item: Kurtz 11

### Sections: 504.7.7.1 and 504.8.7.1

#### Committee Action: Accept in Principle

Comment: Revise text as follows:

**503.2 FASTENER CORROSION RESISTANCE.** Fasteners shall be corrosion resistant meeting ~~ASTM A 641, Class 1~~ complying with 304.3 or an equal corrosion resistance by coating, electro galvanization, mechanical galvanization, hot dipped galvanization, stainless steel, nonferrous metal and alloys or other suitable corrosion-resistant material.

#### Exceptions:

1. Fasteners used to secure roof sheathing.
2. Fasteners for concrete and clay roof tiles shall comply with Table 504.3.5, Note 8.

#### **504.7.7.1 Fasteners** (Page 151)

**504.7.7.1.1 Nails.** Nails to attach the wood shakes shall be 3d stainless steel ring shank nails. The nails shall have sufficient length to penetrate through the wood shakes and shall penetrate through the sheathing.

**504.7.7.1.2 Screws:** Screws to attach the battens to the framing shall be No. 8 by 2-½ inches long corrosion resistant wood screws. Wood screws shall be corrosion resistant screws conforming to ANSI/ASME B 18.6.1. The corrosion resistance shall comply with section 304.3. ~~meet ASTM A 641, Class 1 or an equal corrosion resistance by coating, electro galvanization, mechanical galvanization, stainless steel, nonferrous metal or other suitable corrosion resistant material.~~

**504.7.7.2 Wood Battens:** 1 x 4 wood battens shall be attached to the wood joists with 2 screws per joist. The first batten was located 6 inches from the outer edge of the wood joist. Second batten shall be spaced 1-¼ inches from the first batten. The remaining battens shall be spaced a maximum 2 inches apart, except the last one which shall be spaced no greater than ¾ inches from the previous batten.

#### **504.8.7.1 Fasteners** (Page 153)

**504.8.7.1.1 Nails:** Nails to attach the wood shakes shall be 6d stainless steel ring shank nails. The nails shall have sufficient length to penetrate through the wood shakes and shall penetrate through the sheathing.

**504.8.7.1.2 Screws:** Screws to attach the battens to the framing shall be No. 8 by 2-½ inches long corrosion resistant wood screws. Wood screws shall be corrosion resistant screws conforming to ANSI/ASME B 18.6.1. The corrosion resistance shall comply with section 304.3. ~~meet ASTM A 641, Class 1 or an equal corrosion resistance by coating, electro galvanization, mechanical galvanization, stainless steel, nonferrous metal or other suitable corrosion resistant material.~~

**504.8.7.2 Wood Battens:** 1 x 6 wood battens shall be attached to the wood joists with 2 screws per joist. The first batten was located 6 inches from the outer edge of the wood joist. Second batten shall be spaced 1-¼ inches from the first batten. The remaining battens shall be spaced a maximum 2 inches apart, except the last one which shall be spaced no greater than ¾ inches from the previous batten.

**504.8.7.3 Shakes:** Shakes shall be attached to the battens with 2 nails for each shake placed 1-½ inch above the exposure line. The nails shall be ¾ to 1 inch from the shake edges.

**504.8.8 Shake Placement:** The starter course at the eaves shall be doubled

**Item: Anderson 06**

**Page: 157**

**Committee Action: Negative**

**Comment:** Based on action on Kelly 12

**Item: Kelly 12**

**Page: 157**

**Committee Action: Accept**

**Comment:**

**Item: Barbera 04**

**Table: 602A**

**Committee Action: Accept**

**Amend – add def of “Effective wind area” from ASCE 7 in standard.**

**Add footnote “c” to Table 602A defining effective wind area as follows:**

c. The effective wind area is the span length multiplied by an effective width that need not be less than one-third the span length. For cladding fasteners, the effective wind area shall not be greater than the area that is tributary to an individual fastener.

**Comment: Editorial.**

**Item: Ehrlich 11**

**Table: 602C**

**Committee Action: Accept**

**Comment:**

**Item: Anderson 01**

**Section: 602.1.2**

**Committee Action: Accept**

**Comment:**

**Item: Anderson 02**

**Section: 602.2.2.2**

**Committee Action: Negative**

**Comment:** This is not required. The compilation of the standard may or may not allow for specific order of figures relative to text.

**Item: Anderson 03 – Withdrawn**

**Section: 602.4.4**

**Committee Action:**

**Comment:**

**Item: Anderson 04**

**Section: 603.1**

**Committee Action: Accept**

**Amend:** Also fix in second paragraph of 603.1

**Comment:** Editorial

**Item: Anderson 05**

**Section: 603.2**

**Committee Action: Accept**

**Comment:**

**Item: Hetzel 01**

**Section: 603.2**

**Committee Action: Negative**

**Comment:** This issue is covered in the standard scope. This would be contrary to original scope of standard.

**Item: Herrenbruck 01**

**Section: 701.2**

**Committee Action: Negative**

**Comment:** The proposed language would make this provision of this standard optional.

**Item: Kelly 10**

**Section: 701.2**

**Committee Action: Accept**

**Comment:**

**Item: Herrenbruck 03**

**Section: 701.4**

**Committee Action: Accept**

**Comment:**

**Item: Kelly 11**

**Section: 701.4**

**Committee Action: Accept**

**Comment:**

**Item: Kurtz 12**

**Section: 702.4**

**Committee Action: Accept in Principle**

**Comment:** Accept first part, negative on second part.  
This table is based upon the use of nails, not staples.

**Item: Dobson 01**

**Table: 702.4**

**Committee Action: Negative**

**Comment:** Committee feels that solid sheathing is necessary, and that staples should not be used, in high wind areas.. Also, some level of debris impact resistance is needed. Consistency with latest version of IRC is necessary, but it is not clear that this would provide consistency.

**Item: Herrenbruck 05**

**Table: 702.4**

**Committee Action: Negative --**

**Comment:** See above action and comments on Dobson 01 .Also foam sheathing is not referred to in ASTM D3679.

**Item: Herrenbruck 06**

**Table: 702.4**

**Committee Action: Negative**

**Comment:** See above comments on Dobson 01 and Herrenbruck 05.

**Item: Kurtz 13**

**Section: 702.4**

**Committee Action: Accept in principle**

**Comment:** Modify the table to reference footnote d as suggested. Make no modifications to footnote b. This footnote is consistent with the IRC.

**Item: Schmid 07**

**Section: 703**

**Committee Action: Negative**

**Comment:** This is covered in referenced standards. The details suggested are unnecessary.

**Item: Ehrlich 12**

**Table: 704**

**Committee Action: Accept in Principle**

**Comment:** Modify the table to read as follows:

**TABLE 704  
METAL TIES FOR BRICK VENEER**

<b>Metal Tie Type</b>	<b>Minimum Thickness (Gage)</b>	<b>Maximum Spacing (in.)</b>
Rectangular (Box)	0.148 inches (9)	18" o.c. vert. and 32" horiz.
Ladder or Truss	0.148 inches (9)	18" o.c. vert.
Adjustable Ladder	0.148 inches (9)	18" o.c. vert. and 32" horiz.
Corrugated	0.0299 inches (22)	18" o.c. vert. and 32" horiz.

**Item: Herrenbruck 04**

**Section: 704**

**Committee Action: Negative**

**Comment:** The committee believes that these details are unnecessary in this context.

**Item: Herrenbruck 07**

**Section: 704 and Figure 704**

**Committee Action: Negative**

**Comment:** See Herrenbruck 04.

**Item: Ehrlich 13**

**Section: 704.4**

**Committee Action: Accept**

**Comment:**

**Item: Dobson 02**

**Section: 705**

**Committee Action: Accept**

**Comment:**

**Item: Gardner 01**

**Table: 702.4**

**Committee Action: Negative Pass U**

**Comment:** This construction is not desirable in high wind, and no technical justification was provided to support this. The capacity of gypsum wallboard is unknown.

**Item: Kurtz 14**

**Appendix: A**

**Committee Action: Accept**

**Comment:**

**Item: Schmid 08**

**Appendix: A**

**Committee Action: Negative**

**Comment:** The Reference is already in the standard.

**Item: Schmid 04**

**Appendix: B**

**Committee Action: Negative**

**Comment:** The committee believes that this information is necessary for users of the standard to better understand the basis for the prescriptive details. The appendix is an informational appendix and not part of the consensus standard.

**Item: Ehrlich 14**

**Appendix: C**

**Committee Action: Negative**

**Comment:** See Low 02

**Item: Low 02**

**Appendix: C**

**Committee Action: Accept -Combined with Low 01**

**Comment:** These sections will need to be made applicable to light-framed steel construction.

**Item: Schmid 05**

**Appendix: C**

**Committee Action: Negative**

**Comment:** Has been deleted by Low 02

**Item: Schmid 06**

**Appendix: D**

**Committee Action: Negative**

**Comment:** See Schmid 04.