2009 International Building Code Errata

(Portions of text and tables not shown are unaffected by the errata)

THIRD PRINTING (Updated December 14, 2010)

CHAPTER 18
SOILS AND FOUNDATIONS

Revise TABLE 1807.1.6.3(3) as follows:

MAXIMUM WALL HEIGHT (feet-inches)	MACIMUM UNBALANCES BACKFILL HEIGHT[d] (feet-inches)	MINIMUM VERTICAL REINFORCEMENT-BAR SIZE AND SPACING (inches)		
		Design lateral soil load[a] (psf per foot of depth)		
		30[e]	45[e]	60
8-0	4-0 (or less)	#4 at 56	#4 at 56	#4 at 56
	5-0	#4 at 56	#4 at 56	#4 at 56
	6-0	#4 at 56	#4 at 56	#5 at 56
	7-0	#4 at 56	#5 at 56	#6 at 56
	8-0	#5 at 56	#6 at 56	#7 at 56

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SECOND PRINTING (Updated March 22, 2010)

CHAPTER 18
SOILS AND FOUNDATIONS

1807.1.6.3 Masonry foundation walls. Masonry foundation walls shall comply with the following:

1. & 2*. (No change)*
3. The specified location of the reinforcement shall equal or exceed the effective depth distance, *d*, noted in Tables 1807.1.6.3(2), 1807.1.6.3(3) and 1807.1.6.3(4) and shall be measured from the face of the exterior (soil) side of the wall to the center of the vertical reinforcement. The reinforcement shall be placed within the tolerances specified in TMS 602/ACI 530.1/ASCE 6, Article ~~3.3.B.8~~ 3.4.B.8 of the specified location.

4. through 10. *(No change)*

1810.3.3.1.6 Uplift capacity of grouped deep foundation elements. For grouped deep foundation elements subjected to uplift, the allowableworking uplift load for the group shall be calculated by an *approved* method of analysis. W~~w~~here the deep foundation elements in the group are placed at a center-to-center spacing of at least 2.5 times the least horizontal dimension of the largest single element, the allowable working uplift load for the group is permitted to be calculated as the lesser of:

1. The proposed individual uplift working load times the number of elements in the group.
2. Two-thirds of the effective weight of the group and the soil contained within a block defined by the perimeter of the group and the length of the element.

1810.3.9.1 Design cracking moment. *(No change to text)*

$\Phi M_n = 3\sqrt{f'_c} \times S_m$ **(Equaion 18-11)** *(Note: S_m should not be included under the square root symbol, just f'_c)*

For SI:
$\Phi M_n = 0.25\sqrt{f'_c} \times S_m$