Errata

(Portions of text and tables not shown are unaffected by the errata)

SEVENTH PRINTING (Updated October 13, 2014)

CHAPTER 16 STRUCTURAL DESIGN

Figure 1608.2 GROUND SNOW LOADS, pg, FOR THE UNITED STATES (psf) (Revise southern California values as indicated in the figure)



Errata

(Portions of text and tables not shown are unaffected by the errata)

SIXTH PRINTING (Updated June 24, 2014)

CHAPTER 16 STRUCTURAL DESIGN

1615.4.2.2 Transverse ties. Transverse ties shall consist of continuous reinforcement in slabs; continuous or spliced decks or sheathing; continuous or spliced members framing to, within or across walls; or connections of continuous framing members to walls. Transverse ties shall be placed no farther apart than the spacing of load-bearing walls. Transverse ties shall have minimum nominal tensile strength *TT*, given by Equation 16-46-41. For ASD the minimum nominal tensile strength shall be permitted to be taken as 1.5 times the allowable tensile stress times the area of the tie.

Errata

(Portions of text and tables not shown are unaffected by the errata)

FOURTH PRINTING (Updated Nov. 12, 2013)

CHAPTER 16 STRUCTURAL DESIGN

1613.3.4 Design spectral response acceleration parameters. Five-percent damped design spectral response acceleration at short periods, SDS, and at 1-second period, SDS SD1, shall be determined from Equations 16-39 and 16-40, respectively:

 $S_{DS} = 2/3 S_{MS}$ (Equation 16-39) $S_{D1} = 2/3 S_{M1}$ (Equation 16-40)

where:

 S_{MS} = The maximum considered earthquake spectral response accelerations for short period as determined in Section 1613.3.3.

 S_{M1} = The maximum considered earthquake spectral response accelerations for 1-second period as determined in Section 1613.3.3.

Errata

(Portions of text and tables not shown are unaffected by the errata)

SECOND PRINTING (Updated February 6, 2013)

CHAPTER 16 STRUCTURAL DESIGN

1603.1.3 Roof snow load data. The ground snow load, *Pg*, shall be indicated. In areas where the ground snow load, *Pg*, exceeds 10 pounds per square foot (psf) (0.479 kN/m2), the following additional information shall also be provided, regardless of whether snow loads govern the design of the roof:

- 1. Flat-roof snow load, Pf.
- 2. Snow exposure factor, Ce.
- 3. Snow load importance factor, Is. (Add subscript 's' to Item 3)
- 4. Thermal factor, Ct.

1603.1.4 Wind design data. The following information related to wind loads shall be shown, regardless of whether wind loads govern the design of the lateral-force resisting system of the structure:

- 1. Ultimate design wind speed, V_{ult}, (3-second gust), miles per hour (km/hr) and nominal design wind speed, V_{asd} , as determined in accordance with Section 1609.3.1.
- 2. Risk category.
- 3. Wind exposure: applicable wind direction if .Where more than one wind exposure is utilized, the wind exposure and applicable wind direction shall be indicated.
- 4. The applicable Applicable internal pressure coefficient.
- Components and cladding. The design <u>Design</u> wind pressures in terms of psf (kN/m2) to be used for the design of exterior component and cladding materials not specifically designed by the *registered design* professional responsible for the design of the structure, psf (kN/m²).

1604.8.2 Structural walls. Walls that provide vertical load-bearing resistance or lateral shear resistance for a portion of the structure shall be anchored to the roof and to all floors and members that provide lateral support for the wall or that are supported by the wall. The connections shall be capable of resisting the horizontal forces specified in Section 1.4.4 1.4.5 of ASCE 7 for walls of structures assigned to Seismic Design Category A and to Section 12.11 of ASCE 7 for walls of structures assigned to all other seismic design categories. Required anchors in masonry walls of hollow units or cavity walls shall be embedded in a reinforced grouted structural element of the wall. See Sections 1609 for wind design requirements and 1613 for earthquake design requirements.

1606.1 General. Dead loads are those loads defined in Section <u>1602.1</u> <u>202</u>. Dead loads shall be considered permanent loads.

TABLE 1607.1

MINIMUM UNIFORMLY DISTRIBUTED LIVE LOADS, Lo, AND MINIMUM CONCENTRATED LIVE LOADS_g

Revise item 26 to read as follows: OCCUPANCY OR USE UNIFORM CONCENTRA TED (lbs.) (psf) 26. Roofs 300 All roof surfaces subject to maintenance workers Awnings and canopies: Fabric construction supported by a skeleton structure 5 All other construction nonreducibl Ordinary flat, pitched, and curved roofs (that are not е

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	, j	
occupiable)	20	
Where Primary roof members are exposed to a work floor.,	20	
at		
Single panel point of lower chord of roof trusses or any		2,000
point along primary structural members supporting roofs-		300
over manufacturing, storage warehouses, and repair		
garages		
All other primary roof members		
Occupiable roofs:	100	Note 1
Roof gardens	100 ^m	
Assembly areas	Note 1	
All other similar areas		

(No changes to portions of table not shown or footnotes a-k and m)

I. Areas of occupiable roofs, other than roof gardens and assembly areas, shall be designed for appropriate loads as approved by the building official. Unoccupied landscaped areas of roofs shall be designed in accordance with Section 1607.12.3-1607.12.3.1.

1607.8.2 Grab bars, shower seats and dressing room bench seats. Grab bars, shower seats and dressing room bench seat systems seats shall be designed to resist a single concentrated load of 250 pounds (1.11 kN) applied in any direction at any point on the grab bar or seat so as to produce the maximum load effects.

FIGURE 1608.2 GROUND SNOW LOADS, p_g , FOR THE UNITED STATES (psf)

Revise map as follows:

North Dakota by changing ground snow load value from 36 to 35. Pennsylvania, for 30 psf ground snow load change elevation from (700) to (1700) – 2 locations.

1609.1.2.1 Louvers. Louvers protecting intake and exhaust ventilation ducts not assumed to be open that are located within 30 feet (9144 mm) of grade shall meet requirements of AMCA 54 540.

TABLE 1609.6.2 NET PRESSURE COEFFICIENTS

Revise table as follows:

For "3.Components and cladding in areas of discontinuities-roofs and overhangs", under "Gable or hipped configurations at ridges, eaves and rakes", Flat<Slope<6:12, Positive, 100 square feet or more, under "Partially enclosed" revise table entry from 10.72 to 0.72.

For "4.Components and cladding not in areas of discontinuities-walls and parapets", revise first row to read: "Wall Elements: $h \le 60$ feet (Zone 4) ASCE 7 Figure 30.4-1"; revise sixth row to read: "Wall Elements: h > 60 feet (Zone 4) ASCE 7 Figure 30.6-1"

For "5.Components and cladding in areas of discontinuities-walls and parapets", revise sixth row to read: "Wall Elements: h > 60 feet (Zone 4) ASCE 7 Figure 30.6-1"

1609.6.4.1 Main wind-force-resisting systems. The MWFRS shall be investigated for the torsional effects identified in ASCE 7 Figure <u>27.4.6</u> <u>27.4-8</u>.

1613.3.5 Determination of seismic design category. Structures classified as *Risk Category* I, II or III that are located where the mapped spectral response acceleration parameter at 1-second period, *S1*, is greater than or

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equal to 0.75 shall be assigned to Seismic Design Category E. Structures classified as Risk Category IV that are located where the mapped spectral response acceleration parameter at 1-second period, S1, is greater

than or equal to 0.75 shall be assigned to *Seismic Design Category* F. All other structures shall be assigned to a *seismic design category* based on their *risk category* and the design spectral response acceleration parameters, SDS and SD1, determined in accordance with Section 1613.3.4 or the site-specific procedures of ASCE 7. Each building and structure shall be assigned to the more severe *seismic design category* in accordance with Table 1613.3.5(1) or <u>1613.3.5(2)</u> 1613.5.5(2), irrespective of the fundamental period of vibration of the structure, <u>T.</u>